

MMOS

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CONSULTING CIVIL & STRUCTURAL ENGINEERS

Engineering Services Report Student Accommodation, Rialto Cinema Site, Rialto

Rev. 6

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Project Reference 18_079

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| Revision | Date | Issue | Author | Checked By |
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| 1 | 19.10.18 | Issue for planning review | DOS | MM |
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| 4 | 14.02.19 | Planning review comments added | DOS | MM |
| 5 | 19.04.19 | IW & DCC comments rec'd and report amended | DOS | MM |
| 6 | 04.07.19 | Issued for Planning | DOS | MM |

1.0 Introduction

This Civil Engineering report is to be read in conjunction with the Civil Engineering planning drawings and relevant architects' drawings.

2.0 Site Location and Description of Proposal

The site for the proposed scheme is Old Rialto Cinema site in Rialto, Dublin 8. The site fronts on to the South Circular Road on its northern side. It has the end of a housing terrace and associated rear garden and a single storey small industrial unit on its eastern side. There are open public footpath and hardstanding areas to the south and west sides of the site. The site is currently bordered by a circa 3m height block wall with security fencing over. The overall site area is 0.299Ha. See figures 1 & 2 for site location and plan view. The cinema was constructed in circa 1930's and operated as a cinema until the early 1970's. It was then converted to a Car Showroom. The site has been unoccupied for several years.

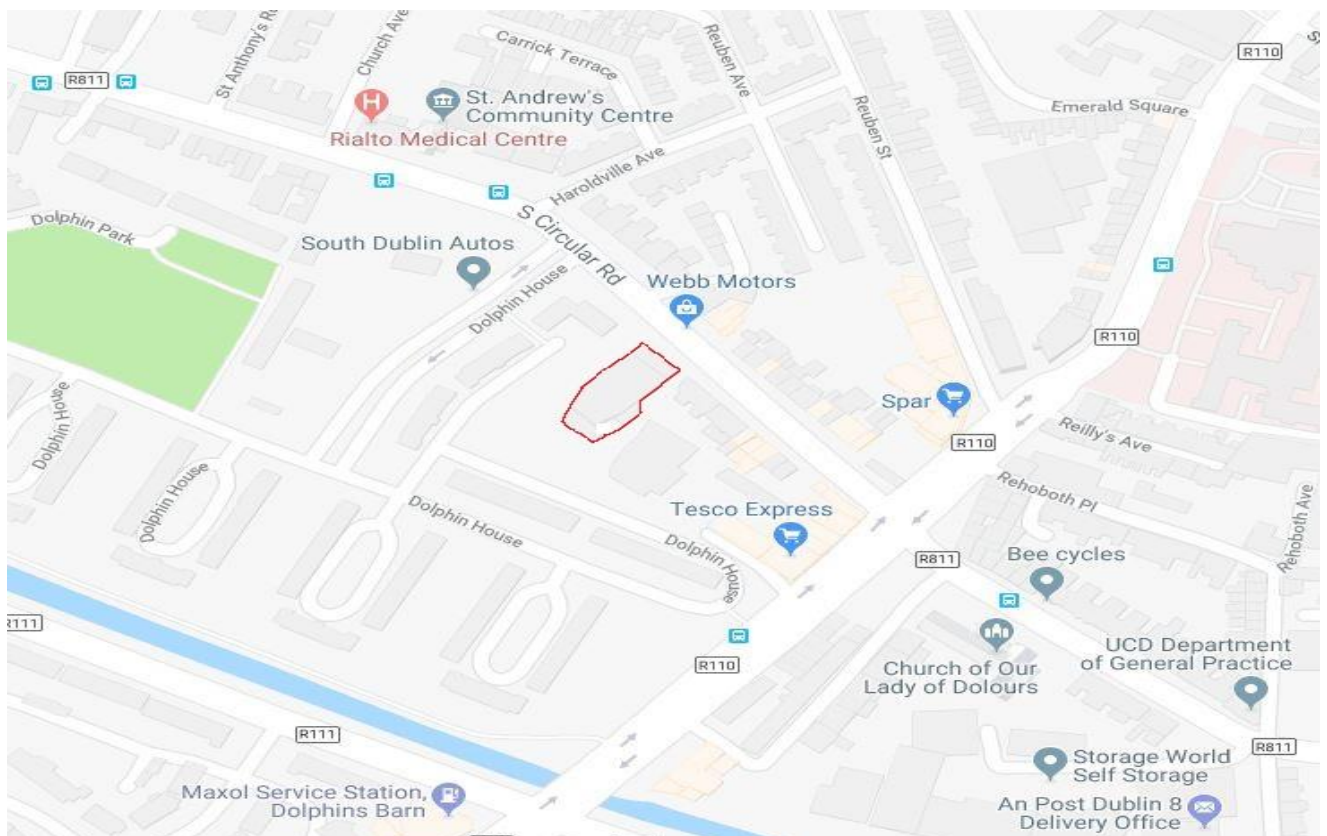


Figure 1 – Site Location Plan



Figure 2 – Site Plan view

The proposed development consists of a 5-7 storey above ground new Student Accommodation facility, with amenity and servicing areas at lower ground floor level. Accommodation is for circa 317 no. students (313 bedrooms) with associated facilities. The proposed development is to incorporate part of the existing 3 storey old Cinema structure to the front of the site. The remainder rear of the existing building on site will be demolished. Refer to figs 3 & 4 for Lower & Ground Floor levels -1 & 0 & architect's drawings attached to this application for full details.

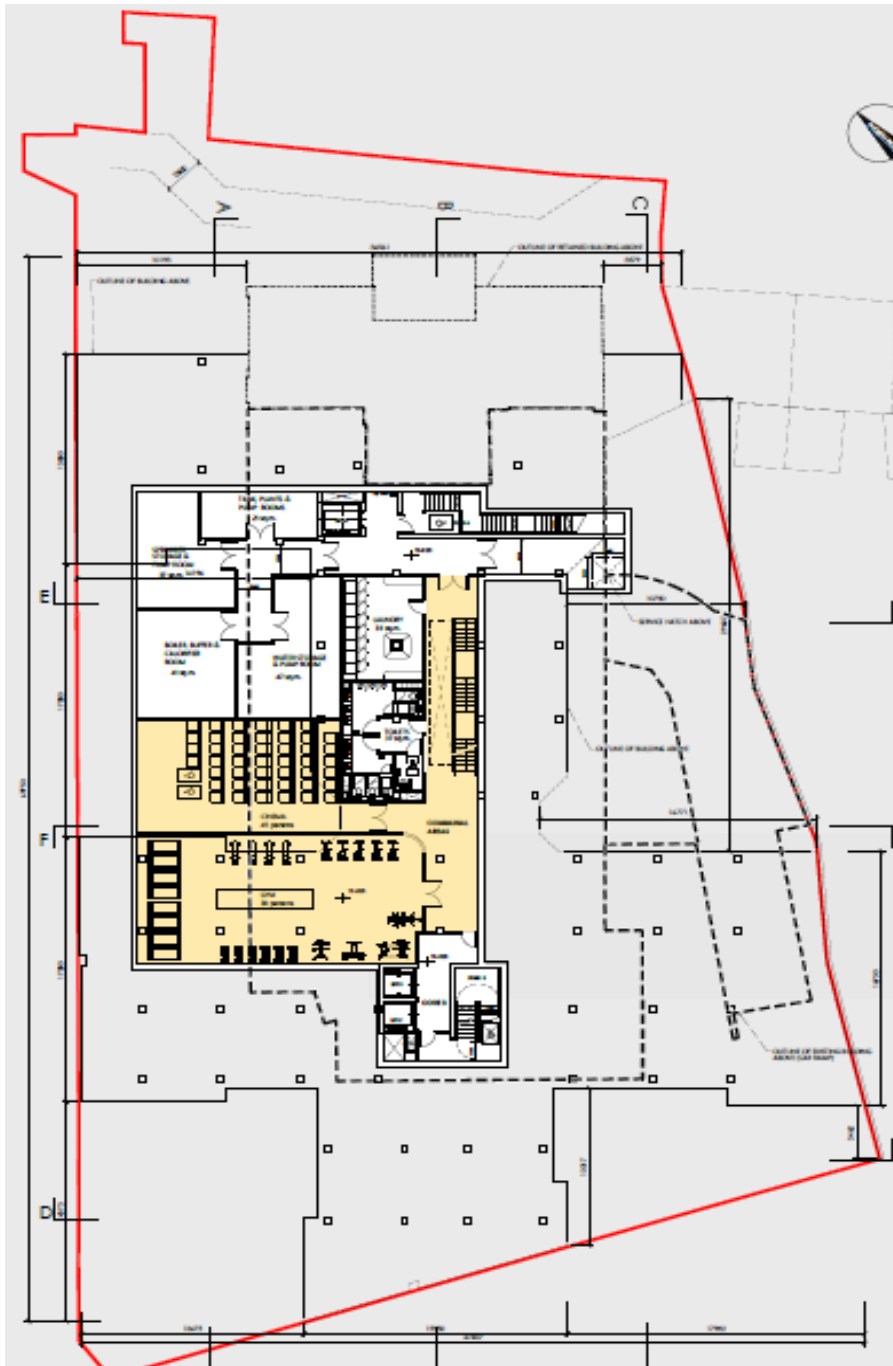


Fig. 3 - Lower Ground Floor Level -1



Fig. 4 - Ground Floor Level 0

The existing site levels vary from +19.50m AOD at the front of the site rising to +20.40m AOD at the rear. The proposed Upper Ground floor level is at +19.95m (Level 0 – see fig. 4) and the Lower Ground Floor level is at +15.825m (Level -1 – See fig. 03). The proposed scheme has the lowest level of building being constructed circa 4.6m below ground (Level -1 – Lower Ground Floor area containing Leisure facilities and general building plant rooms. The proposed development incorporates 3no. External Courtyard areas (courtyards 2, 3 & 4) at circa +19.90m AOD). There is no requirement for car parking within the site. Only courtyard 2 is accessible to vehicle, for emergency access by fire tenders in case of fire.

3.0 Surface Water Discharge

Existing Surface Water Drainage

At present the existing building and hardstanding surrounding area drains by gravity to either a combined public sewer in the South Circular Road or a surface water sewer with an outfall manhole on the pavement directly northwest of the site. From IW/DCC records the combined sewer is circa 600mm wide x 1030mm ht. brick culvert which runs by gravity from west to east. The surface water sewer is a 225mm diam. sewer running from south to north across the South Circular Road. See fig. 5 below.

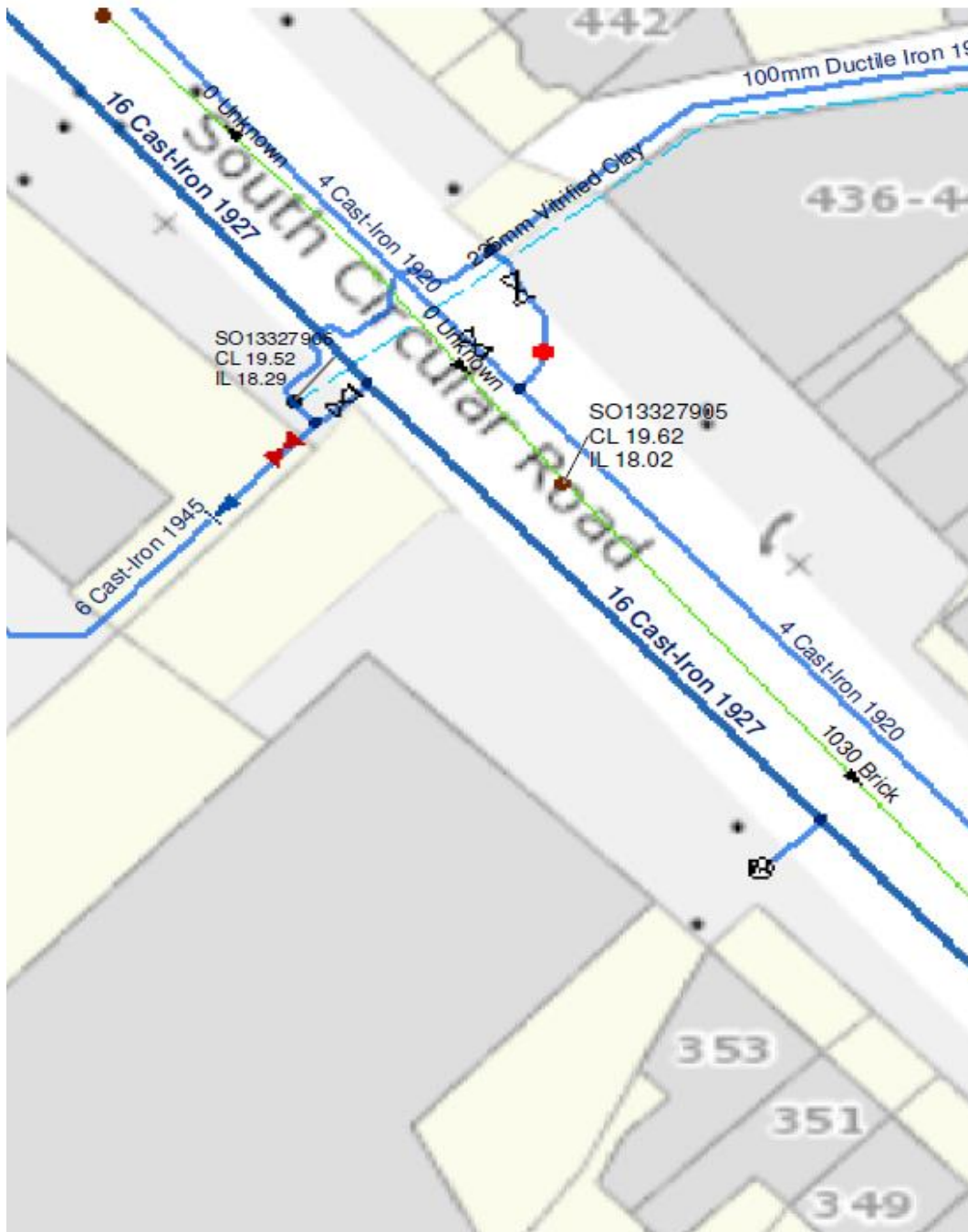


Fig 5 – Irish Water records extract

Proposed Surface Water Drainage

Storm water from the upper roof areas will drain, via RW outlets, gullies, downpipes and suspended SW drainage pipework, to a gravity network of below ground surface water sewers on the perimeter of the site at Upper Ground floor level. These sewers will drain by gravity to an onsite attenuation facility proposed on the eastern side of the site. Attenuation capacity is designed for a 1 in 100 year storm event + 20% allowance for climate change. Attenuation will be provided by a 75m³ below ground storage tank. Attenuated outfall from this system will fall by gravity to the existing surface water sewer via the existing manhole in the pavement directly north west of the site. Surface water outfall from the attenuation tank is to be restricted by a hydrobrake. The small site area (0.297 Ha.) gives a theoretical greenfield run-off rate less than 2 l/s and as such a 2 l/s value was used to calculate the required attenuation storage volume. The peak stormwater discharge is therefore to be restricted to 2 l/s (or as per lowest commercially available restriction hydrobrake requirement).

Non-return valves shall be incorporated into the SW system at points of entry of the gravity sewers into the attenuation tank. Note also that a relieving gully outlet at ground level will be built within the attenuation tank design to allow outfall should exceedance occur.

Surface Water Drainage Design – Pipe Sizing

Site is split with SW drain runs on each side, each taking circa 50% of site area

Check outfall pipe for ¼ of site area

Contributing Area = 750m² max.

$Q = 0.075 \times 75 \times 2.78 = 15.6 \text{ l/s}$

Add 20% for climate change, $Q = 18.77 \text{ l/s}$

For 225mm diam. @ 1/200, $K_s = 0.6\text{mm}$

Capacity = 36.5 l/s 225mm diam. pipe OK

Check outfall pipe for 1/2 of site area

$Q = 37.54 \text{ l/s}$

For 300mm diam. @ 1/200, $K_s = 0.6\text{mm}$

Capacity = 78.2 l/s 300mm diam. pipe OK

4.0 SuDS

Sustainable Drainage System

The Greater Dublin Strategic Drainage Study (GDSDS) Vol. 2 Section E2.1 requires provision of interception and/or treatment volume for River Water Quality Protection. It is intended to provide interception for first 5-10mm of rainfall within the site by incorporation of Green Roof to approx. 70% of total new roof area and inherent permeable paving within the courtyard areas. Refer to Shipsey Barry Architects roof plan drawing no. SB-2017-015-206 and Fig. 6 below for details of extent of Green Roof. Refer to attached MMOS drawings 18079 – 1000 & 1007 for details. The following 'soft' SuDS measures are proposed within the site;

- **Green Roofs system to upper roofs**

It is intended to provide an overall area of Green Roof of just under 70% of new roof areas (1,264m²). The system proposed for majority of green roof area will incorporate a Sedum type Blanket over a Bauder DSE40 water retention/drainage and protection layer green roof system (or similar approved). This is an Extensive type of green roof which provides a water storage capacity of 13.5 Litres/m² and will provide interception storage for the first 5-10mm of rainfall. As well as improving water quality by providing surface water interception and infiltration, green roofs improve biodiversity by providing habitat for wildlife. Refer to attached Sedum Mats and Bauder DSE40 Data sheets for details.

- **Permeable paving to courtyards 2, 3 & 4**

It is intended to provide permeable paving (pervious paving and porous asphalt) within the courtyard areas (circa 650m²) to provide Interception in the courtyard paving and landscaping area by means of provide inherent storage capacity within the voided sub-base of the Courtyard area at Level 0. This will act to trap suspended solid and filter pollutants from stormwater

The above measure will provide for interception treatment by infiltration for a total area of 1914m² or 64% of total development site area (Total site area = 2,973m²).

Additional SuDs measures proposed within the site are detailed below;

- **Water Butts and Rainwater Harvesting** – a small local water butt at Ground level and larger Rainwater Harvesting storage tank at basement level are to be provided within the site to provide non-potable water storage for serving landscaping and general irrigation and maintenance needs. Rainwater harvesting will reduce the

quantity of rainwater entering the surface water system and reduce the water demand on mains water supply.

- **Attenuation** – Attenuation is to be provided by means of a below ground Storage Tank to restrict outflow from the development to 2 l/sec/Hectare, in accordance with the requirements of the Greater Dublin Strategic Drainage Study (GDSDS). Attenuation is to be designed for a 1 in 100 year storm event + 20% for climate change. The attenuation volume required is 75m³.

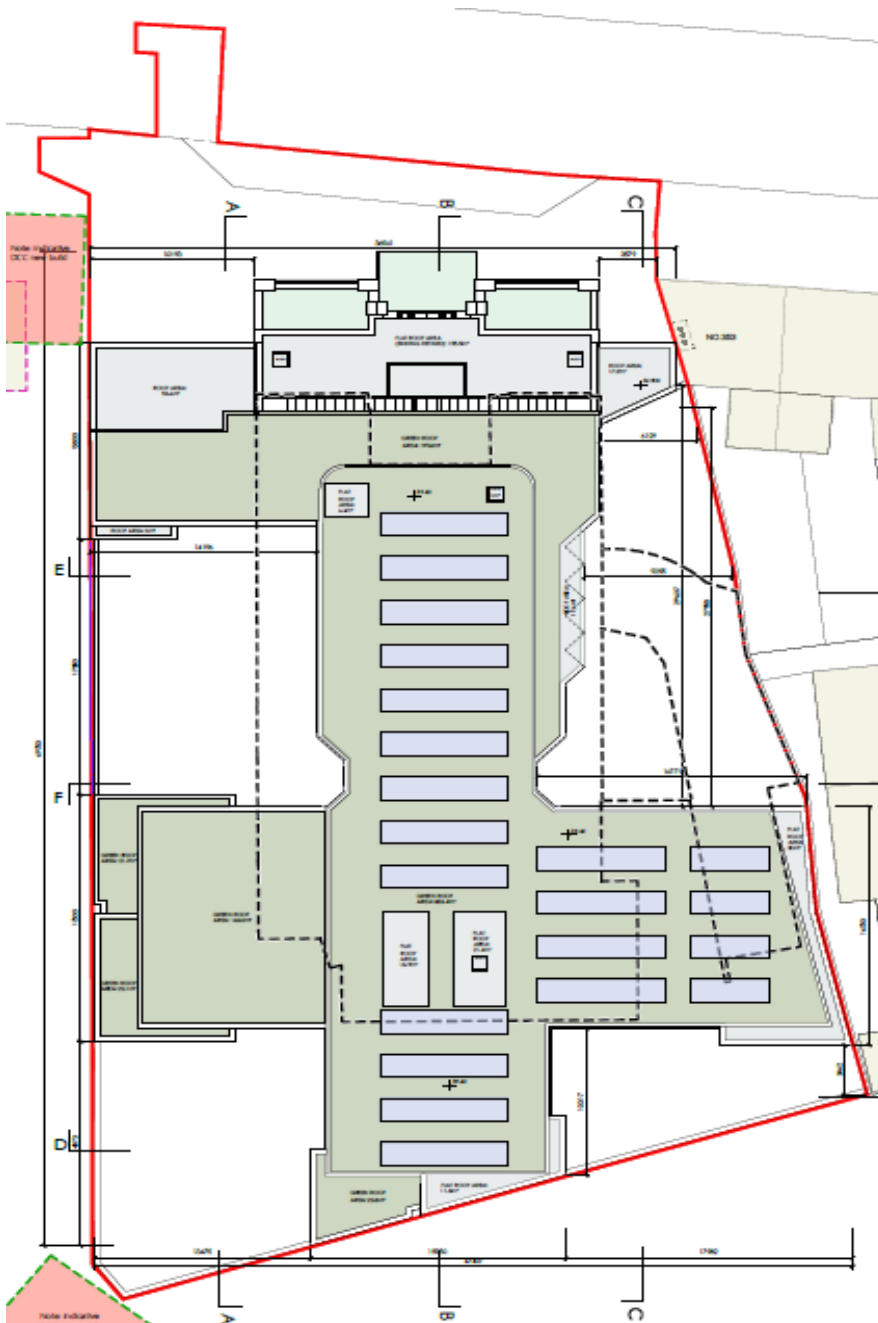


Fig. 6 – Roof plan with Green roof areas highlighted green

5.0 Foul Water Discharge

There is an existing foul sewer network on site serving the cinema and latterly the Motor company. This will be mostly removed as part of the demolition works and replaced by a new Foul Drainage system which will largely discharge from the accommodation units at each level, via vertical shafts, to a foul water collection system at Upper Ground Floor level (Level 0). Refer to drawings 18079-1000 & 1006 for details. As the Lower Ground Floor level (Level -1) is below the invert of the combined sewer in the South Circular Road, a Foul Water pumping station is required at this level, for any outfall foul drainage from the leisure facilities at this level, with duty and stand-by pumps and with volume of chamber sized for 24 hour storage of foul water discharging at level -1. This Foul outfall will be pumped to a transition manhole on site at Upper Ground floor level and then fall by gravity within the main site foul sewer system to the combined sewer in South Circular Road.

The foul water outfall is as follows;

Per Capita Consumption = 150 l/head/day

Occupancy Rate = 1 (1 person per student unit)

Average Day Peak multiplier = 1.25

Unaccounted for Water = 110 l/conn./day

(note; 1 no. connection proposed for development)

The Per Property (Student Unit) Domestic Demand = $150 \times 1 \times 1.25$

= 187.5 l/head/day

For 317 no. Student Accommodation Units

= $187.5 \times 317 + (110) = 59,547.5$ litres per day

Therefore DWF = $59,547.5 / 24 \times 60 \times 60 = 0.690$ litres/ sec.

6 x DWF = 4.135 litres / sec.

Foul Sewer Pipe Sizing

Peak flow = 4.135 l/s

By inspection use min. 150mm diam. pipe at 1/150 min. fall

Capacity = 11.5 l/s => Pipe okay by inspection

6.0 Water Supply

There are 2no. public watermains within the South Circular Road. It is intended to take a 150mm branch connection from the existing 100mm diam. watermain on the north side of the South Circular Road and directly in front of the proposed development (as per Irish Water requirements). Refer to drawings 18079 – 1000 & 1008 for details. The connection will be metered and fire fighting requirements for the site will be subject to detailed design and agreement with the local authority.

Refer to section 5.0 above for total water demand. The proposed development will require 59.55m³/day. An onsite water storage tank will be provided to satisfy the 24hr water storage requirement.

Summary of Results for 100 year Return Period (+20%)

Half Drain Time : 377 minutes.


| Storm Event | Max Level (m) | Max Depth (m) | Max Infiltration (l/s) | Max Control (l/s) | Max E Outflow (l/s) | Max Volume (m ³) | Status |
|------------------|---------------|---------------|------------------------|-------------------|---------------------|------------------------------|--------|
| 15 min Summer | 12.818 | 0.443 | 0.0 | 1.5 | 1.5 | 29.2 | O K |
| 30 min Summer | 12.971 | 0.596 | 0.0 | 1.5 | 1.5 | 39.3 | O K |
| 60 min Summer | 13.112 | 0.737 | 0.0 | 1.6 | 1.6 | 48.6 | O K |
| 120 min Summer | 13.239 | 0.864 | 0.0 | 1.7 | 1.7 | 57.0 | O K |
| 180 min Summer | 13.294 | 0.919 | 0.0 | 1.8 | 1.8 | 60.7 | O K |
| 240 min Summer | 13.320 | 0.945 | 0.0 | 1.8 | 1.8 | 62.3 | O K |
| 360 min Summer | 13.335 | 0.960 | 0.0 | 1.8 | 1.8 | 63.4 | O K |
| 480 min Summer | 13.338 | 0.963 | 0.0 | 1.8 | 1.8 | 63.5 | O K |
| 600 min Summer | 13.333 | 0.958 | 0.0 | 1.8 | 1.8 | 63.2 | O K |
| 720 min Summer | 13.324 | 0.949 | 0.0 | 1.8 | 1.8 | 62.7 | O K |
| 960 min Summer | 13.301 | 0.926 | 0.0 | 1.8 | 1.8 | 61.1 | O K |
| 1440 min Summer | 13.246 | 0.871 | 0.0 | 1.7 | 1.7 | 57.5 | O K |
| 2160 min Summer | 13.161 | 0.786 | 0.0 | 1.6 | 1.6 | 51.9 | O K |
| 2880 min Summer | 13.081 | 0.706 | 0.0 | 1.5 | 1.5 | 46.6 | O K |
| 4320 min Summer | 12.938 | 0.563 | 0.0 | 1.5 | 1.5 | 37.2 | O K |
| 5760 min Summer | 12.820 | 0.445 | 0.0 | 1.5 | 1.5 | 29.4 | O K |
| 7200 min Summer | 12.719 | 0.344 | 0.0 | 1.5 | 1.5 | 22.7 | O K |
| 8640 min Summer | 12.519 | 0.144 | 0.0 | 1.5 | 1.5 | 9.5 | O K |
| 10080 min Summer | 12.497 | 0.122 | 0.0 | 1.4 | 1.4 | 8.0 | O K |
| 15 min Winter | 12.872 | 0.497 | 0.0 | 1.5 | 1.5 | 32.8 | O K |

| Storm Event | Rain (mm/hr) | Flooded Volume (m ³) | Discharge Volume (m ³) | Time-Peak (mins) |
|------------------|--------------|----------------------------------|------------------------------------|------------------|
| 15 min Summer | 91.990 | 0.0 | 30.3 | 26 |
| 30 min Summer | 62.658 | 0.0 | 41.4 | 40 |
| 60 min Summer | 39.995 | 0.0 | 53.0 | 68 |
| 120 min Summer | 24.904 | 0.0 | 66.0 | 126 |
| 180 min Summer | 18.717 | 0.0 | 74.4 | 182 |
| 240 min Summer | 15.262 | 0.0 | 80.9 | 240 |
| 360 min Summer | 11.412 | 0.0 | 90.8 | 304 |
| 480 min Summer | 9.274 | 0.0 | 98.4 | 368 |
| 600 min Summer | 7.890 | 0.0 | 104.6 | 434 |
| 720 min Summer | 6.913 | 0.0 | 110.0 | 502 |
| 960 min Summer | 5.608 | 0.0 | 119.0 | 644 |
| 1440 min Summer | 4.175 | 0.0 | 132.8 | 918 |
| 2160 min Summer | 3.107 | 0.0 | 148.5 | 1324 |
| 2880 min Summer | 2.518 | 0.0 | 160.4 | 1732 |
| 4320 min Summer | 1.870 | 0.0 | 178.6 | 2512 |
| 5760 min Summer | 1.512 | 0.0 | 192.7 | 3288 |
| 7200 min Summer | 1.283 | 0.0 | 204.3 | 4048 |
| 8640 min Summer | 1.121 | 0.0 | 214.2 | 4424 |
| 10080 min Summer | 1.000 | 0.0 | 222.9 | 5144 |
| 15 min Winter | 91.990 | 0.0 | 34.0 | 26 |

Summary of Results for 100 year Return Period (+20%)

| Storm Event | Max Level (m) | Max Depth (m) | Max Infiltration (l/s) | Max Control (l/s) | Max Σ Outflow (l/s) | Max Volume (m ³) | Status |
|-----------------------|---------------|---------------|------------------------|-------------------|---------------------|------------------------------|------------|
| 30 min Winter | 13.045 | 0.670 | 0.0 | 1.5 | 1.5 | 44.2 | O K |
| 60 min Winter | 13.207 | 0.832 | 0.0 | 1.7 | 1.7 | 54.9 | O K |
| 120 min Winter | 13.357 | 0.982 | 0.0 | 1.8 | 1.8 | 64.8 | O K |
| 180 min Winter | 13.426 | 1.051 | 0.0 | 1.9 | 1.9 | 69.4 | O K |
| 240 min Winter | 13.462 | 1.087 | 0.0 | 1.9 | 1.9 | 71.7 | O K |
| 360 min Winter | 13.483 | 1.108 | 0.0 | 1.9 | 1.9 | 73.2 | O K |
| 480 min Winter | 13.483 | 1.108 | 0.0 | 1.9 | 1.9 | 73.2 | O K |
| 600 min Winter | 13.477 | 1.102 | 0.0 | 1.9 | 1.9 | 72.7 | O K |
| 720 min Winter | 13.463 | 1.088 | 0.0 | 1.9 | 1.9 | 71.8 | O K |
| 960 min Winter | 13.424 | 1.049 | 0.0 | 1.9 | 1.9 | 69.2 | O K |
| 1440 min Winter | 13.333 | 0.958 | 0.0 | 1.8 | 1.8 | 63.2 | O K |
| 2160 min Winter | 13.200 | 0.825 | 0.0 | 1.7 | 1.7 | 54.4 | O K |
| 2880 min Winter | 13.079 | 0.704 | 0.0 | 1.5 | 1.5 | 46.5 | O K |
| 4320 min Winter | 12.875 | 0.500 | 0.0 | 1.5 | 1.5 | 33.0 | O K |
| 5760 min Winter | 12.523 | 0.148 | 0.0 | 1.5 | 1.5 | 9.8 | O K |
| 7200 min Winter | 12.487 | 0.112 | 0.0 | 1.3 | 1.3 | 7.4 | O K |
| 8640 min Winter | 12.469 | 0.094 | 0.0 | 1.2 | 1.2 | 6.2 | O K |
| 10080 min Winter | 12.456 | 0.081 | 0.0 | 1.0 | 1.0 | 5.4 | O K |

| Storm Event | Rain (mm/hr) | Flooded Volume (m ³) | Discharge Volume (m ³) | Time-Peak (mins) |
|-----------------------|--------------|----------------------------------|------------------------------------|------------------|
| 30 min Winter | 62.658 | 0.0 | 46.4 | 39 |
| 60 min Winter | 39.995 | 0.0 | 59.4 | 68 |
| 120 min Winter | 24.904 | 0.0 | 74.0 | 124 |
| 180 min Winter | 18.717 | 0.0 | 83.4 | 180 |
| 240 min Winter | 15.262 | 0.0 | 90.7 | 236 |
| 360 min Winter | 11.412 | 0.0 | 101.7 | 340 |
| 480 min Winter | 9.274 | 0.0 | 110.2 | 384 |
| 600 min Winter | 7.890 | 0.0 | 117.2 | 462 |
| 720 min Winter | 6.913 | 0.0 | 123.2 | 540 |
| 960 min Winter | 5.608 | 0.0 | 133.3 | 694 |
| 1440 min Winter | 4.175 | 0.0 | 148.8 | 990 |
| 2160 min Winter | 3.107 | 0.0 | 166.3 | 1416 |
| 2880 min Winter | 2.518 | 0.0 | 179.6 | 1828 |
| 4320 min Winter | 1.870 | 0.0 | 200.0 | 2648 |
| 5760 min Winter | 1.512 | 0.0 | 215.8 | 3064 |
| 7200 min Winter | 1.283 | 0.0 | 228.8 | 3744 |
| 8640 min Winter | 1.121 | 0.0 | 239.9 | 4416 |
| 10080 min Winter | 1.000 | 0.0 | 249.7 | 5144 |

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Rainfall Details

| | | | |
|-----------------------|----------------------|-----------------------|-------|
| Rainfall Model | FSR | Winter Storms | Yes |
| Return Period (years) | 100 | Cv (Summer) | 0.750 |
| Region | Scotland and Ireland | Cv (Winter) | 0.840 |
| M5-60 (mm) | 17.000 | Shortest Storm (mins) | 15 |
| Ratio R | 0.300 | Longest Storm (mins) | 10080 |
| Summer Storms | Yes | Climate Change % | +20 |

Time Area Diagram

Total Area (ha) 0.177

| Time (mins) | | Area | Time (mins) | | Area | Time (mins) | | Area |
|-------------|-----|-------|-------------|-----|-------|-------------|-----|-------|
| From: | To: | (ha) | From: | To: | (ha) | From: | To: | (ha) |
| 0 | 4 | 0.059 | 4 | 8 | 0.059 | 8 | 12 | 0.059 |

TECHNICAL DATA SHEET**Bauder DSE40 Drainage and Protection Layer****DESCRIPTION:**

Water storage and multi-directional drainage layer that provides a pressure resistant stable base for high loads or support for roof mounted equipment without compression to the drainage capacity.

TECHNICAL DATA:**Composition**

Material Recycled High Density Polyethylene

Weights and sizes

Size: 1.04m x 2.03m

Thickness: 40mm

Coverage: 2.1m²

Weight: 1.8kg/m²

Saturated Weight: 15.3kg water only/27kg infilled with mineral drain

Water Storage Capacity: 13.5l/m² empty/8.4l/m² infilled with mineral drain

Fill Volume (Mineral Drain): 21l/m²

Compressive Strength: 80kN/m² when empty/≥ 1000kN/m² when infilled

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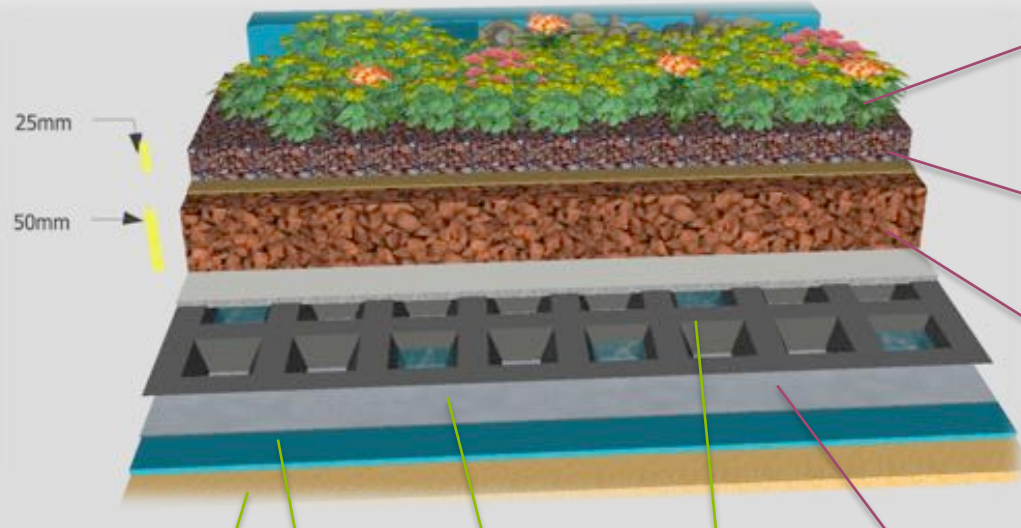
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Structural plywood roof

Waterproof membrane

Protects your roof

Root barrier

Filter Fleece

Ensures efficient water and nutrient distribution and retention, enhancing growth and

Water retention and

- Saturated weight – 98Kg/m²
- 10-20mm thickness range
- Flexible polyethylene

